

## Modelling the influence of total suspended solids on *E. coli* removal in river water

Jueying Qian, Evelyn Walters, Peter Rutschmann, Michael Wagner and Harald Horn

### ABSTRACT

Following sewer overflows, fecal indicator bacteria enter surface waters and may experience different lysis or growth processes. A 1D mathematical model was developed to predict total suspended solids (TSS) and *Escherichia coli* concentrations based on field measurements in a large-scale flume system simulating a combined sewer overflow. The removal mechanisms of natural inactivation, UV inactivation, and sedimentation were modelled. For the sedimentation process, one, two or three particle size classes were incorporated separately into the model. Moreover, the UV sensitivity coefficient  $\alpha$  and natural inactivation coefficient  $k_d$  were both formulated as functions of TSS concentration. It was observed that the *E. coli* removal was predicted more accurately by incorporating two particle size classes. However, addition of a third particle size class only improved the model slightly. When  $\alpha$  and  $k_d$  were allowed to vary with the TSS concentration, the model was able to predict *E. coli* fate and transport at different TSS concentrations accurately and flexibly. A sensitivity analysis revealed that the mechanisms of UV and natural inactivation were more influential at low TSS concentrations, whereas the sedimentation process became more important at elevated TSS concentrations.

**Key words** | *E. coli*, modelling, particle size distribution, removal mechanism

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### INTRODUCTION

Surface water quality is closely related to the quality of life in a city. In Europe, the bathing water quality is usually satisfactory according to the EU bathing water directive (EEA 2015). However, after periods of intense rain the water quality is expected to be non-compliant due to microbiological contamination. This is because heavy rain events are often associated with municipal sewer overflows, bacteria resuspension, and surface run-off (Davies & Bavor 2000; Nagels *et al.* 2002; Garzio-Hadzick *et al.* 2010). It is thus important to determine how fast the waters achieve sufficient quality again. Fecal indicator bacteria (FIB) such as fecal coliforms (FC), *Escherichia coli* (*E. coli*), and *Enterococcus* are accepted indicators of microbiological contamination and can be used to evaluate the microbial water quality after rainfall.

After entering surface waters, FIB may experience growth, lysis, predation, and UV inactivation. Researchers

studied the survival rate of FIB in different physical and chemical conditions, e.g. sunlight, pH value, temperature, turbidity, salinity, hydraulic parameters, and nutrient concentrations, to evaluate their continued risks to human health (Evison 1989; Pommepuy *et al.* 1992; Canteras *et al.* 1995; Howell *et al.* 1996; Sinton *et al.* 2002; Kay *et al.* 2005; Schultz-Fademrecht *et al.* 2008; Garzio-Hadzick *et al.* 2010; Walters *et al.* 2014a). FIB may also attach to particles, or remain freely suspended in the water body (Characklis *et al.* 2005). Particle-associated bacteria may settle out of the bulk liquid and thus render sedimentation a considerable removal mechanism.

In an FIB fate and transport model, the particle-associated bacteria are commonly assumed to have the same settling velocity as the particles they are associated with. Traditionally and for the reason of simplification, one

median particle size is often chosen and incorporated in models to simulate the sedimentation process (Steets & Holden 2003; Cho et al. 2012; Ghimire & Deng 2013). However, the particle size of sediment varies over two to three orders of magnitude and the settling velocity varies over four to six orders of magnitude (Ghimire & Deng 2013). So far, little attention has been paid to the influence of the particle size distribution in FIB removal models. Therefore, it is likely that the precision of water quality models can be further improved by incorporating different particle size classes into them.

Suspended solids do not only influence the settling characteristics of bacteria. It is reported that the existence of suspended solids/particles increases the survival of coliforms in bulk liquid (Qualls et al. 1985). Moreover, compared to freely suspended bacteria, particle-associated bacteria are known to have higher survival rates. Possible reasons are (i) particles shield the bacteria from UV irradiation, (ii) the pores of the particles protect the FIB from predation (Wright et al. 1995; Decamp & Warren 2000), (iii) the particles offer a better micro-environment with availability of nutrients and substrates, and (iv) certain sediments (i.e. smectite clays) enhance the formation of protective biofilms (Jamieson et al. 2005; Alimova et al. 2007).

As far as we are aware, a study relating bacteria removal from the water column with particle (total suspended solids) concentrations as well as particle size distribution has to date not been reported. In this study, an *E. coli* fate and transport model was developed and calibrated using experimental data from a large-scale experimental setup (Walters et al. 2014b). The primary objectives of this study were (i) to study the impact of the particle size distribution on the removal of FIB, (ii) to assess the impact of sedimentation, UV and natural inactivation as removal mechanisms, and (iii) to study the relationship between total suspended solids (TSS) concentration and FIB removal from the water column.

## MATERIALS AND METHODS

### Large-scale flume setup

A detailed description of the experimental setup has been published in Walters et al. (2014b). Briefly, the flume was built of concrete (length = 12 m, width = 0.5 m, water depth  $h = 0.5$  m,  $V \approx 14$  m<sup>3</sup>) and was operated either in flow-through or recirculation mode. In recirculation mode,

four pumps transported water from the collection basin back to the inlet basin.

The water used in the experiments was from the Oberrach River (Bavaria, Germany; side stream of the Isar River). The oligotrophic Oberrach water was directed to a reservoir to allow large particles to settle. Gravels taken from the Isar River ( $d_{50} = 15\text{--}30$  mm) were spread in the flume bed to mimic a natural river environment. Before each experiment the flume was operated in flow-through mode for a minimum of 3 weeks to allow fine particles to settle.

### Experiment A

This experiment was performed in order to investigate FIB removal from the water column of an oligotrophic river following a combined sewer overflow (CSO). The CSO was simulated by addition of raw wastewater ( $V = 1$  m<sup>3</sup>) from the municipal wastewater treatment plant in Garching (Bavaria, Germany) to the flume operated in recirculation mode. The ratio of wastewater and river water was 1:13 (V:V) and the volumetric flow rate was constant at  $0.2$  m<sup>3</sup> s<sup>-1</sup>; experimental duration was 70 hours. This experiment is referred to as Experiment 2 in the original publication of Walters et al. (2014b).

### Experiment B

To investigate the impact of resuspended sediment on the FIB removal rate, the flume was initially operated at a volumetric flow rate of  $0.1$  m<sup>3</sup> s<sup>-1</sup> with  $1$  m<sup>3</sup> of wastewater added to  $13$  m<sup>3</sup> of river water as in Experiment A. After 140 min the volumetric flow rate was doubled to induce resuspension of settled particles. The experiment duration was 70 hours. This experiment is referred to as Experiment 3 in the original publication of Walters et al. (2014b).

### Sampling procedure of Experiments A and B

Uniform sampling for *E. coli* enumeration and TSS measurement always occurred in the middle of the flume (length of 6 m) at predefined time interval by a team of researchers (for details see Walters et al. (2014b)). Samples were stored at 4 °C and analysed within 24 hours. Viable *E. coli* were enumerated using the standardized microplate methods for surface water, DIN EN ISO 9308-3 (Bio-Rad, Munich, Germany), based on the presence of the beta-glucuronidase enzyme. The average surface UV irradiance in daytime in Experiments A and B was  $6.5$  W m<sup>-2</sup> and  $17$  W m<sup>-2</sup>, respectively; the average water temperature in Experiments

A and B was 15.7 and 19.7 °C. Parameters used within the simulations are summarized in Table 1.

In addition to the coarse gravel, fine sediments were also present on the flume bed. These fine sediments were a mixture of particles originating from the river (water) and raw wastewater. The particle size distribution of the fine sediments was measured after the experiments by sieving and filtering techniques (Walters 2013). The Oberrach River is an oligotrophic river and has very low concentrations of *E. coli*. Background *E. coli* concentrations ranged between 0.26 and 5.87 most probable number (MPN) ml<sup>-1</sup>. Most of the *E. coli* in the experiment thus originated from the added raw wastewater. The partitioning behaviour of *E. coli* to different particle sizes in wastewater was characterized by Walters et al. (2013) and are listed in Table 2. Jeng et al. (2005) found comparable *E. coli* partitioning characteristics in urban storm water run-off. Therefore, it was assumed that the association of *E. coli* to different particle size classes in the flume was identical to that of *E. coli* to wastewater particles measured by Walters et al. (2013).

## Modelling process

A model was developed to describe the fate and transport of solid particles/TSS and *E. coli* in a flume mimicking an oligotrophic river following a CSO. Model setup, parameter estimations (chi-squared test), and a sensitivity analysis (absolute-relative sensitivity function) were performed using AQUASIM (version 2.1d, Reichert (1994)). The model is 1D in the vertical direction and considers dispersion as well as reactions. The Reynolds number in the flume was  $>5 \times 10^5$  and the dispersion coefficient was calculated to be 180 m<sup>2</sup> h<sup>-1</sup> (Jobson & Sayre 1970) when the volumetric flow rate reached 0.2 m<sup>3</sup> s<sup>-1</sup>. Flow was therefore regarded as being turbulent and the system was thus treated as a continuous stirred-tank reactor. For simplicity reasons, concentration gradients in the longitudinal direction as well as the influence of the lateral velocity were neglected. This has further been proved by additional simulations (data not shown).

Table 1 | Parameters used in the 1D model

Symbol	Unit	Value/expression	Variable/parameters	Reference
$C_{EC}^0$	MPN m <sup>-3</sup>	Experiment A: $7.3 \times 10^8$ Experiment B: $6.4 \times 10^8$	total initial <i>E. coli</i> concentration	Walters et al. (2014b)
$C_{TSS}^0$	kg m <sup>-3</sup>	Experiment A: 0.015 Experiment B: 0.060	total initial TSS concentration	Walters et al. (2014b)
$d_j$	µm	see Table 2	median diameter of particle class <i>j</i>	Walters (2013)
$D$	m <sup>2</sup> h <sup>-1</sup>	180	dispersion coefficient	Jobson & Sayre (1970)
$F_{sun}$	-	$F_{sun} = 1$ , during daytime, $F_{sun} = 0$ , during nighttime	Boolean variable. It activates or inactivates the mechanism of UV inactivation	measured
$f_{P,j}$	-	see Table 2	<i>E. coli</i> fraction to particle size class <i>j</i>	Walters et al. (2013)
$f_{P,total}$	-	Experiment A: 0.13 Experiment B: 0.58	fraction of total attached <i>E. coli</i>	Walters et al. (2013)
$f_{TSS,j}$	-	see Table 2	percentage of particle class <i>j</i>	Walters (2013)
$h$	m	0.5	water depth	Walters et al. (2014b)
$I_0$	Watt m <sup>-2</sup>	Experiment A: 6.5 Experiment B: 17	average surface UV irradiance	Walters et al. (2014b)
$I_e$	Watt m <sup>-2</sup>	$I_e = \frac{I_0}{k_{att}h} (1 - e^{-k_{att}h})$	average UV irradiance in the epilimnion	Auer & Niemaus (1993)
$m_\alpha$	-	53	parameter of $\alpha$	calibrated
$m_k$	-	0.16	parameter of $k_d$	calibrated
$n_\alpha$	-	-112	parameter of $\alpha$	calibrated
$n_k$	-	-82	parameter of $k_d$	calibrated
$\rho_j$	kg m <sup>-3</sup>	see Table 2	density of particle class <i>j</i>	calibrated
$P$	MPN mgTSS <sup>-1</sup>	6,200	amount of <i>E. coli</i> associated with suspended particles	estimated
$v_j$	m h <sup>-1</sup>	see Table 2	settling velocity of particle class <i>j</i>	Wu & Wang (2006)

**Table 2** | Particle distribution in three cases and *E. coli* fraction to each particle size class

	Case 1	Case 2		Case 3		
	Class i	Class i	Class ii	Class i	Class ii	Class iii
Particle characteristics						
Median diameter ( $\mu\text{m}$ )	23	8	100	8	40	500
Density ( $\text{kg m}^{-3}$ )	1,170	1,450	1,020	1,450	1,100	1,003
Settling velocity ( $\text{m h}^{-1}$ )	0.13	0.04	0.28	0.04	0.22	1.01
Particle distribution in flume water $f_{TSS,j}$						
Experiment A (-)	1	0.17	0.83	0.14	0.60	0.26
Experiment B (-)	1	0.42	0.58	0.42	0.48	0.10
<i>E. coli</i> fraction associated with each particle size class $f_{P,j}$						
Experiment A & B (-)	1	0.04	0.96	0.04	0.88	0.08

The model included a TSS module and an *E. coli* module. In both modules, the influence of different particle size classes was investigated. For Experiment A the simulation started when the wastewater was added to the flume ( $t = 0$ ). Since Experiment B was performed as a resuspension experiment, the simulation began after resuspension was induced by increasing the volumetric flow rate to  $0.2 \text{ m}^3 \text{ s}^{-1}$ . Namely Experiment B simulated how the resuspended particles settled down and how *E. coli* were removed from the water column after resuspension. In the *E. coli* module, neither resuspension nor a transient effect from the bottom sediment zone (Ghimire & Deng 2013; Yakirevich et al. 2013) were considered, because *E. coli* concentrations in the bed sediment before and after adding the wastewater were low (Walters et al. 2014b).

### TSS module

The TSS module describes dispersion and settling processes. Three separate cases have been simulated by incorporating either one, two, or three particle size classes referred to as Case 1, Case 2, and Case 3, respectively. The TSS concentration has been simulated using the following equations:

$$C_{TSS} = \sum_{j=1}^n C_{TSS,j} \quad (1)$$

$$\frac{\partial C_{TSS,j}}{\partial t} = D \frac{\partial^2 C_{TSS,j}}{\partial x^2} - v_j \frac{\partial C_{TSS,j}}{\partial x} \quad (2)$$

where  $n$  is the number of particle size classes,  $C_{TSS}$  is the overall TSS concentration,  $C_{TSS,j}$  is the TSS concentration of particle size class  $j$ , and  $v_j$  is the settling velocity of

particle size class  $j$ . The calculation of settling velocity  $v_j$  involves the particle density and particle size (Wu & Wang 2006),  $D$  is the dispersion coefficient and  $x$  represents the vertical axis. The initial condition of  $C_{TSS,j}$  is described by Equation (3).  $f_{TSS,j}$  is the percentage of particle class  $j$  at  $t = 0$  (Table 2). Equations (4) and (5) represent the lower ( $x_{0.5}$ ) and upper ( $x_0$ ) boundary condition.

$$C_{TSS,j}^0 = f_{TSS,j} C_{TSS}^0 \quad (3)$$

$$D \frac{\partial C_{TSS,j}}{\partial x}(x_{0.5}) = 0 \quad (4)$$

$$D \frac{\partial C_{TSS,j}}{\partial x}(x_0) = 0 \quad (5)$$

### *E. coli* module

The total *E. coli* concentration was partitioned into freely suspended and particle-associated *E. coli*. The particle-associated bacteria were further fractionated into different particle size classes as described in the TSS module for Case 2 and Case 3.

$$C_{EC} = C_F + C_P \quad (6)$$

$$C_P = \sum_{j=1}^n C_{P,j} \quad (7)$$

where  $C_{EC}$  is total *E. coli* concentration,  $C_F$  and  $C_P$  are the concentrations of freely suspended and particle-associated

bacteria.  $C_{P,j}$  represents the concentration of *E. coli* attached to particle size class  $j$ .

*E. coli* has a mass density slightly higher than water (Godin et al. 2007). Thus, the settling velocity of freely suspended *E. coli* was calculated to be more than 50 times lower compared to that of the smallest particle class. Therefore the settling mechanism of freely suspended *E. coli* was neglected and the particle-associated bacteria were assumed to have the same settling velocity as the particles they were associated with. *E. coli* fate and transport equations are presented in Equations (8) and (9).

$$\frac{\partial C_F}{\partial t} = D \frac{\partial^2 C_F}{\partial x^2} - F_{sun} k_{UV} C_F - k_d C_F \quad (8)$$

$$\frac{\partial C_{P,j}}{\partial t} = D \frac{\partial^2 C_{P,j}}{\partial x^2} - v_j \frac{\partial C_{P,j}}{\partial x} - F_{sun} k_{UV} C_{P,j} - k_d C_{P,j} \quad (9)$$

where

$$k_{att} = 0.13 C_{TSS} + 0.27 \quad (10)$$

$$I_e = \frac{I_0}{k_{att} h} (1 - e^{-k_{att} h}) \quad (11)$$

$$k_{UV} = 3600 \alpha I_e \quad (12)$$

$$\alpha = m_\alpha e^{n_\alpha C_{TSS}} \quad (13)$$

$$k_d = m_k e^{n_k C_{TSS}} \quad (14)$$

Note that  $k_d$  is the natural inactivation coefficient and  $k_{UV}$  is the UV inactivation coefficient.  $F_{sun}$  is a Boolean variable being '1' during daytime and '0' during nighttime and activates or deactivates the UV inactivation mechanism.  $k_{att}$  is the average vertical attenuation coefficient. The equation for  $k_{att}$  was determined by additional laboratory experiments using heavily TSS-laden water from the flume system (Walters 2013).  $I_e$  is the depth-average UV irradiance in the water column and was calculated according to Auer & Niemaus (1993).  $I_0$  is the average surface UV irradiance at daytime and  $h$  is the water depth (0.5 m). The UV sensitivity coefficient  $\alpha$  and natural inactivation coefficient  $k_d$  are assumed to be functions of the TSS concentration;  $m_\alpha$ ,  $n_\alpha$ ,  $m_k$ , and  $n_k$  are the parameters of the functions used to correlate  $\alpha$  and  $k_d$  to the TSS concentrations. It is assumed that when the TSS concentration increases,  $\alpha$  and  $k_d$  tend to be zero. Although the decay rates of suspended and

particle-associated bacteria may differ, we assumed only one  $\alpha$  and one  $k_d$ , avoiding a non-essential complexity of the model.

Jamieson et al. (2005) suggest that in a low ionic strength environment such as a freshwater stream, bacteria adsorption would be dominated by strong bonding mechanisms or irreversible attachment. Since the Oberrach is an oligotrophic river, adsorption of *E. coli* to sediment particles was assumed to be irreversible. Initial conditions of the *E. coli* module are listed in Equations (15)–(17).

$$C_P^0 = C_{TSS} P = f_{P,total} C_{EC}^0 \quad (15)$$

$$C_F^0 = (1 - f_{P,total}) C_{EC}^0 \quad (16)$$

$$C_{P,j}^0 = f_{P,j} C_P^0 \quad (17)$$

Note that  $C_{EC}^0$ ,  $C_P^0$ ,  $C_F^0$  and  $C_{P,j}^0$  are the initial concentrations of total *E. coli*, attached *E. coli*, freely suspended *E. coli* and the *E. coli* attached to particle size class  $j$ .  $f_{P,total}$  is the fraction of attached bacteria to total *E. coli* concentration at  $t = 0$ .  $f_{P,j}$  represents the fraction of attached *E. coli* which are associated with particle size class  $j$  at  $t = 0$ .  $P$  is the amount of *E. coli* associated with suspended particles. George et al. (2004) found an approximately linear relationship between the percentage of *E. coli* associated with suspended particles and the TSS concentration. In this study,  $P$  was estimated to equal 6,200 MPN mgTSS<sup>-1</sup>. The resulting fractions of attached bacteria  $f_{P,total}$  are 13% in Experiment A and 58% in Experiment B, respectively. Those values are comparable to the values reported in literature. For example, Characklis et al. (2005) found an average of 20–35% of bacteria associated with particles in background samples and 30–55% in storm water samples. All parameters used within the simulations are summarized in Table 1. The boundary conditions of the *E. coli* module are similar to the TSS module.

## RESULTS

### Model calibration

With the data obtained from the field experiments as described in Walters et al. (2014b) and Walters (2013) a calibration procedure was used to determine the following unknown parameters: particle density of each particle size class  $\rho_j$ , UV sensitivity coefficient  $\alpha$ , and the natural

inactivation coefficient  $k_d$ . Simulated concentrations resemble the concentration at water depth of 0.25 m. Particle density and particle sizes were used to calculate the settling velocities. Estimation of the UV sensitivity coefficient  $\alpha$  and the natural inactivation coefficient  $k_d$  was performed according to Equations (13) and (14), respectively. Firstly, the particle density of each particle size class was varied so that the simulated TSS concentrations closely agreed with the measured TSS concentrations over the experimental duration. Secondly, the optimized particle densities were incorporated into the settling mechanism of the *E. coli* module, and the four parameters  $m_\alpha$ ,  $n_\alpha$ ,  $m_k$ , and  $n_k$  were varied so that the model output closely agreed with the measured *E. coli* concentrations. Calibrated particle densities are summarized in Table 2. The expressions of  $\alpha$  and  $k_d$  are as follows:  $\alpha = 53 e^{-112 C_{TSS}}$  and  $k_d = 0.16 e^{-82 C_{TSS}}$ .

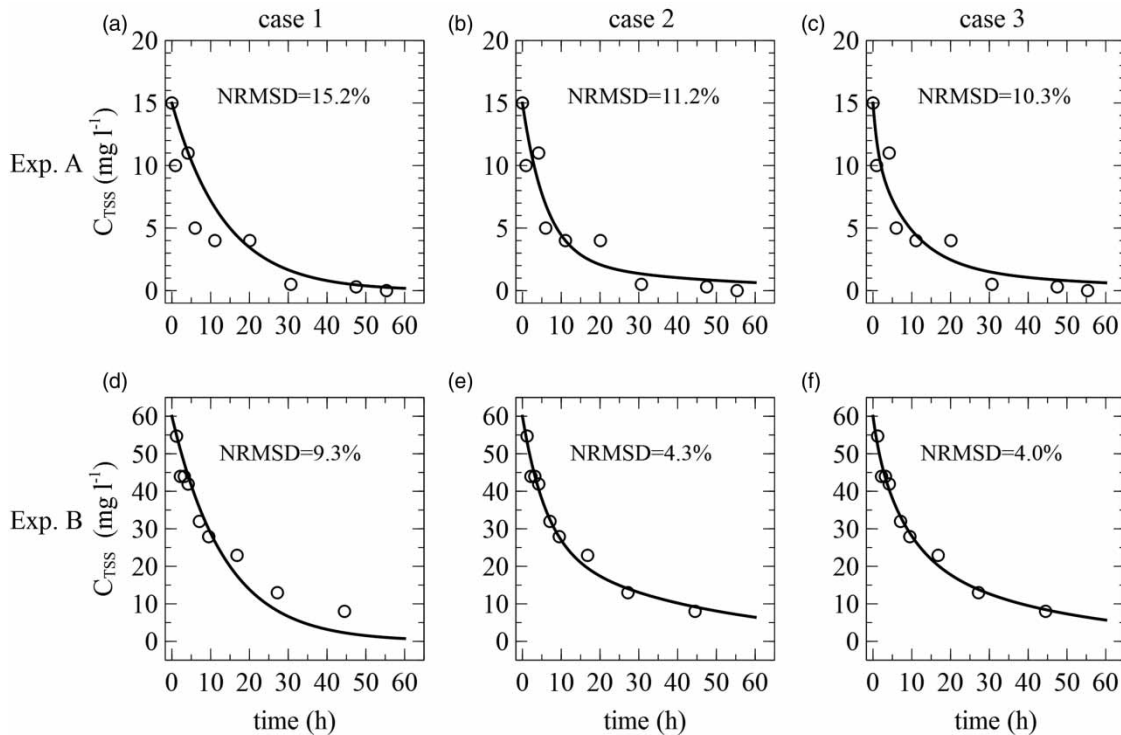
### Effect of particle size distribution on the simulation of TSS and *E. coli* concentrations

Table 2 presents the different particle size classes and properties used to investigate the influence of particle size classes

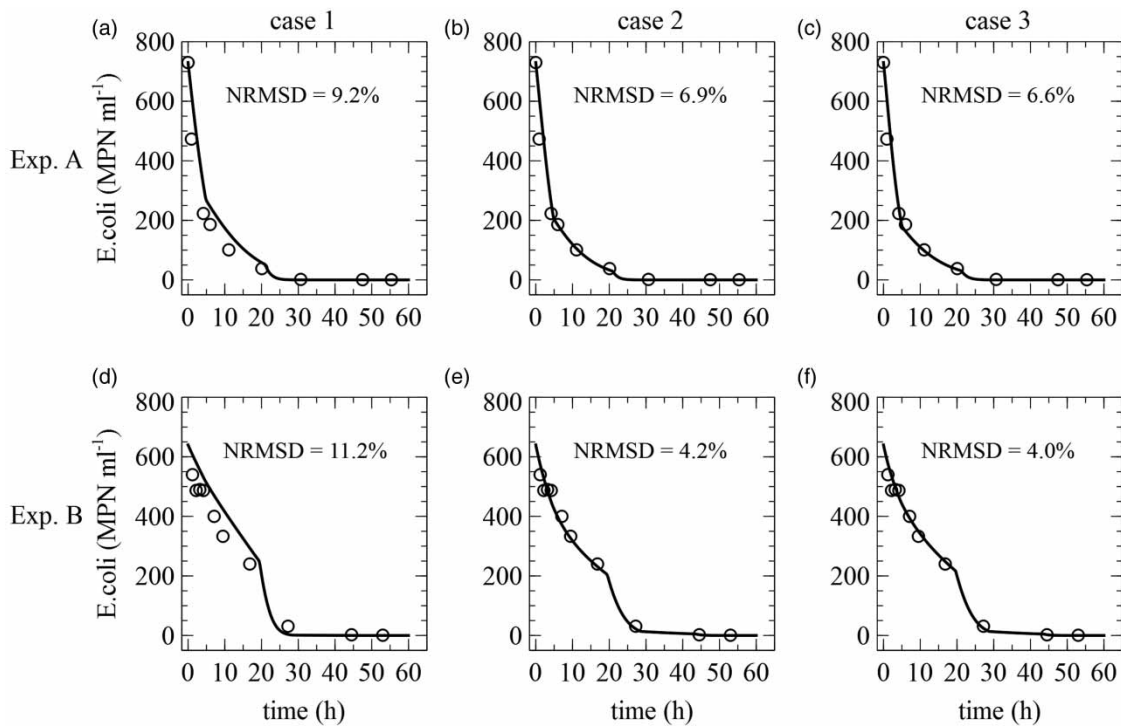
on the model performance. The calibrated densities in Case 2 are as follows:  $\rho_1 = 1,450 \text{ kg m}^{-3}$  ( $d_1 = 8 \mu\text{m}$ ) and  $\rho_2 = 1,020 \text{ kg m}^{-3}$  ( $d_2 = 100 \mu\text{m}$ ). In most mineral soils, the dry density varies from 1,100 to 1,600  $\text{kg m}^{-3}$  (Hillel 1980). Therefore the calibrated density values can be assumed to be acceptable.

The observed and predicted TSS as well as *E. coli* concentrations for the three cases are shown in Figures 1 and 2, respectively. In Figure 1(a) and 1(d) it can be seen that the model does not fit the measured TSS concentrations optimally when only one particle size class was incorporated in the model ( $d_1 = 23 \mu\text{m}$ , see Table 2). For Case 2 (Figure 1(b) and 1(e)) and Case 3 (Figure 1(c) and 1(f)), where more than one particle size class was defined, predictions of the TSS concentrations were obviously enhanced. The normalized root-mean-square deviation (NRMSD) improved from 15.2% (Figure 1(a)) to 10.3% (Figure 1(c)) in Experiment A and from 9.3% (Figure 1(d)) to 4.0% (Figure 1(f)) in Experiment B when three particle size classes were incorporated.

Figure 2 presents the simulation results of *E. coli* for the three cases. Here it can be seen that the particle size distribution has a similar impact on the simulated *E. coli*



**Figure 1** | Influence of the particle size distribution on the simulated TSS concentrations in Experiments A (top row) and B (bottom row). Simulation results depend on either one (a), (d), two (b), (e), or three (c), (f) particle size classes. Measured values are average values of three samples and indicated by circles. NRMSD is normalized root-mean-square deviation quantifying the difference between the measured values and the simulation results.



**Figure 2** | Influence of the particle size distribution on the simulation of *E. coli* concentrations in Experiments A (top row) and B (bottom row). Simulation results depend on either one (a), (d), two (b), (e), or three (c), (f) particle size classes. Measured values are average values of three samples and indicated by circles. NRMSD is normalized root-mean-square deviation.

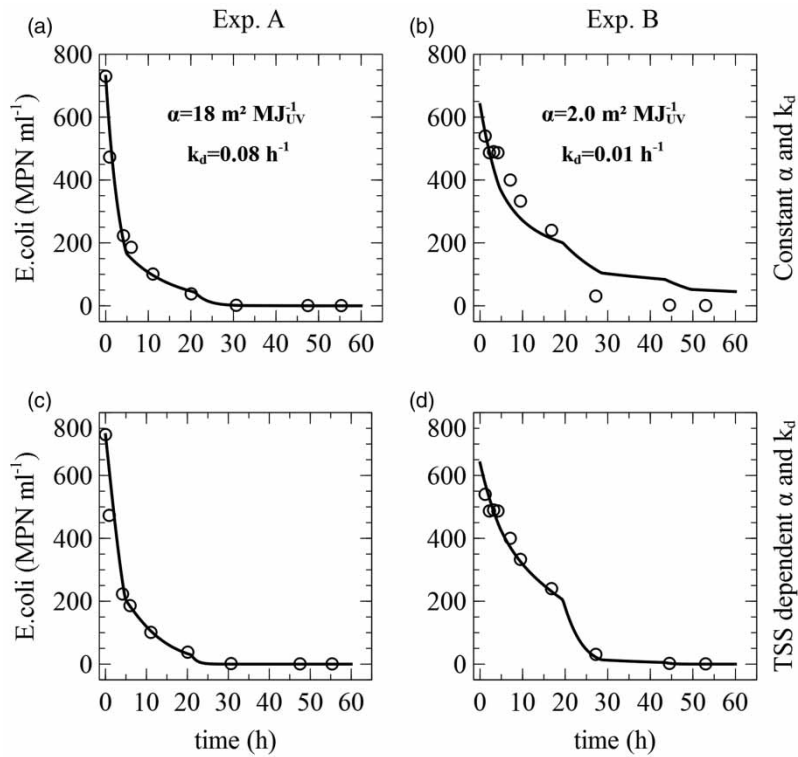
and TSS concentrations. The simulation results for *E. coli* in Cases 2 and 3 fit the experimental data much better than Case 1. Based on the NRMSD, the accuracy of the model using three particle size classes (Case 3) is only slightly improved (less than 1% NRMSD difference) compared to two classes (Case 2) in both experiments. Therefore, all further simulations were performed with only two particle size classes.

### Model improvement by integration of TSS concentration-dependent parameters

Usually  $\alpha$  and  $k_d$  are constants in a model (Jamieson et al. 2005; Cho et al. 2010). This study compared the performance of the model incorporating constant (Figure 3(a) and 3(b)) and TSS concentration-dependent values for  $\alpha$  and  $k_d$  (Figure 3(c) and 3(d)). Figure 3(a) and 3(b) show that the calibrated values of  $\alpha$  ( $18 \text{ m}^2 \text{ MJ}_{\text{UV}}^{-1}$ ) and  $k_d$  ( $0.09 \text{ h}^{-1}$ ) in Experiment A are seven to nine times higher than  $\alpha$  ( $2 \text{ m}^2 \text{ MJ}_{\text{UV}}^{-1}$ ) and  $k_d$  ( $0.01 \text{ h}^{-1}$ ) in Experiment B. This indicates a higher *E. coli* removal rate in Experiment A although the average UV irradiance in Experiment B was twice as high as in Experiment A (see Table 1). It is hypothesized that the sudden increase in

flow rate in Experiment B led to the resuspension of fine bed sediments in the water column, which delayed the *E. coli* removal. A similar hypothesis was proposed by de Brauwere et al. (2011) who found their model underestimating the *E. coli* concentrations in the maximum turbidity zone of an estuary. They suggested that this occurred because the *E. coli* dynamics were modelled independent of the suspended matter.

In order to correlate the *E. coli* removal rate with the TSS concentration,  $\alpha$  and  $k_d$  were assumed to be exponential functions of the TSS concentration in this study. The calibrated results were  $\alpha = 53 e^{-112 C_{\text{TSS}}}$  and  $k_d = 0.16 e^{-82 C_{\text{TSS}}}$  as previously mentioned. Generally, the uniform expressions for  $\alpha$  and  $k_d$  gave a good prediction in both experiments, and simulations with TSS-dependent parameters showed better consonance with the observed values. A more obvious model improvement can be seen in Figure 3(b) and 3(d) for Experiment B compared to Experiment A (see Figure 3(a) and 3(c)). A possible explanation for this trend is that Experiment A had an initial TSS concentration of  $C_{\text{TSS}}^0 = 15 \text{ mg l}^{-1}$ . The variation of the TSS concentration had less influence on  $\alpha$  and  $k_d$  compared to Experiment B where the initial TSS concentration was four times higher ( $C_{\text{TSS}}^0 = 60 \text{ mg l}^{-1}$ ). Therefore, it is

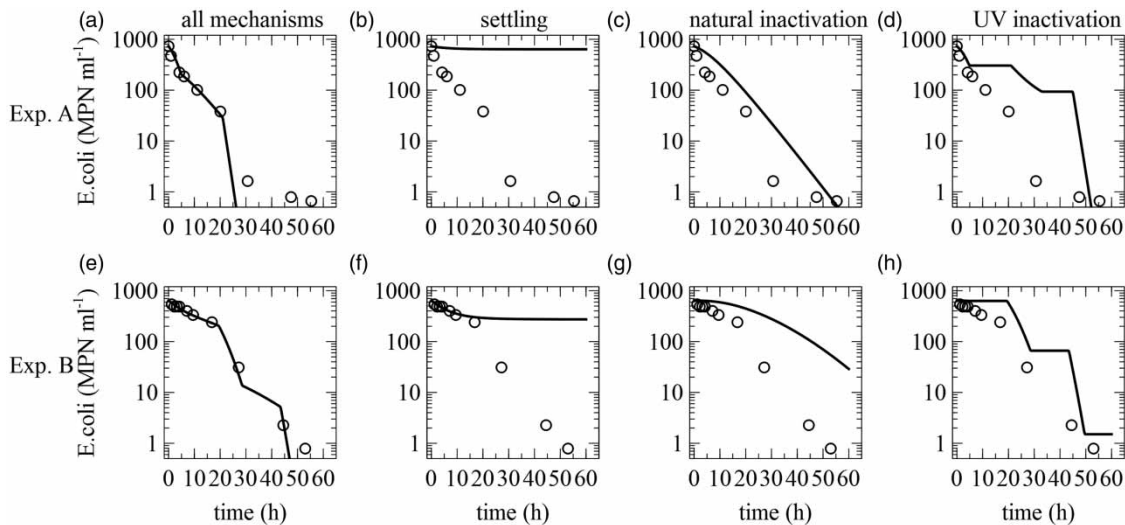


**Figure 3** | Comparison of models incorporating constant parameters and TSS-dependent parameters in both experiments. The top row shows simulation with constant parameters and the bottom with TSS-dependent parameters. Measured values are average values of three samples and indicated by circles.

possible to use constant  $\alpha$  and  $k_d$  in the *E. coli* removal model when the TSS concentration is low or constant. Otherwise, a TSS-dependent parameter will more accurately simulate the UV and natural inactivation processes.

#### Influence of each mechanism on the removal of *E. coli*

Figure 4 shows the influence of each individual mechanism in Experiments A (Figure 4(a)–4(d)) and B (Figure 4(e)–4(h)),



**Figure 4** | Impact of the three different removal mechanisms on the prediction of *E. coli* concentrations in the water column throughout Experiment A (top row) and Experiment B (bottom row). Plots (a) and (e) show the simulation with all three mechanisms, plots (b) and (f) show the impact of the particle settling, plots (c) and (g) show the impact of die-off, plots (d) and (h) show the impact of UV inactivation. Measured values are average values of three samples and indicated by circles.



respectively. Note that Figure 4(a) and 4(e) illustrate the same measured and predicted values as in Figure 3(c) and 3(d), but subfigures in Figure 4 are logarithmically scaled. When settling was the only mechanism considered, UV and natural inactivation coefficients were assumed to equal zero. When only natural or UV inactivation was simulated, the suspended particles were assumed to deposit and *E. coli* were assumed to be completely freely suspended without being influenced by sedimentation.

Figure 4(b) and 4(f) show that the settling mechanism removed bacteria from the water column only at the beginning of the experiments. This is because a large portion of particles settled out of the water column within the first 20 hours (Figure 1). Moreover, the settling mechanism had almost no effect in Experiment A (Figure 4(b)) due to the low TSS concentration and *E. coli* attachment rate. The natural inactivation rate was low at the beginning in Experiment B, due to a higher TSS concentration of 60 mg l<sup>-1</sup>, and increased gradually (Figure 4(g)). In contrast, the natural inactivation rate in Experiment A was relatively high, because of a four times lower initial TSS concentration of 15 mg l<sup>-1</sup> (Figure 4(c)). Solar inactivation only occurs during the daytime, explaining the stepwise reduction seen in Figure 4(d) and 4(h).

### Sensitivity analysis of the *E. coli* concentration prediction

Table 3 shows the ranking of the absolute–relative sensitivity analysis for *E. coli* modelling in both experiments using AQUASIM. Density of particle class II ( $\rho_2$ ) and initial TSS concentration  $C_{TSS}^0$  were the most sensitive parameters in both experiments. Densities of the particles were used to calculate the settling velocity of the particles as well as of *E. coli*. Initial TSS concentrations are essential in this model since the TSS concentration at a later simulation time depends on it. Furthermore, the vertical attenuation coefficient  $k_{att}$ , the UV sensitivity coefficient  $\alpha$ , and the natural inactivation coefficient  $k_d$  are all related to the TSS concentration.

The total percentage of attached *E. coli*,  $f_{P,total}$ , was more sensitive in Experiment B (ranked 2nd) compared to Experiment A (ranked 12th). In this study, 58% of *E. coli* in Experiment B were particle-associated compared to 13% in Experiment A. When more bacteria were attached to particles, the total bacterial concentration decreased more rapidly from the water column due to the settling mechanism. This is the reason for higher sensitivity of  $f_{P,total}$  in Experiment B. The density of particle class I ( $\rho_1$ ) ranked 10th and 4th in Experiments A and B, respectively.

Table 3 | Sensitivity analysis results for *E. coli* simulation

Ranking	Experiment A	Experiment B
1	$\rho_2$	$\rho_2$
2	$C_{TSS}^0$	$f_{P,total}$
3	$f_{TSS,2}$	$C_{TSS}^0$
4	$n_\alpha$	$\rho_1$
5	$I_0$	$f_{TSS,1}$
6	$m_\alpha$	$n_k$
7	$m_k$	$f_{P,2}$
8	$n_k$	$n_\alpha$
9	$f_{TSS,1}$	$m_k$
10	$\rho_1$	$f_{TSS,2}$
11	$f_{P,2}$	$f_{P,total}$
12	$f_{P,total}$	$m_\alpha$
13	$f_{P,1}$	$f_{P,1}$
14	$D$	$D$

Since both  $f_{P,total}$  and  $\rho_1$  were more sensitive in Experiment B, it can be concluded that the settling process plays a more important role at elevated TSS concentrations.

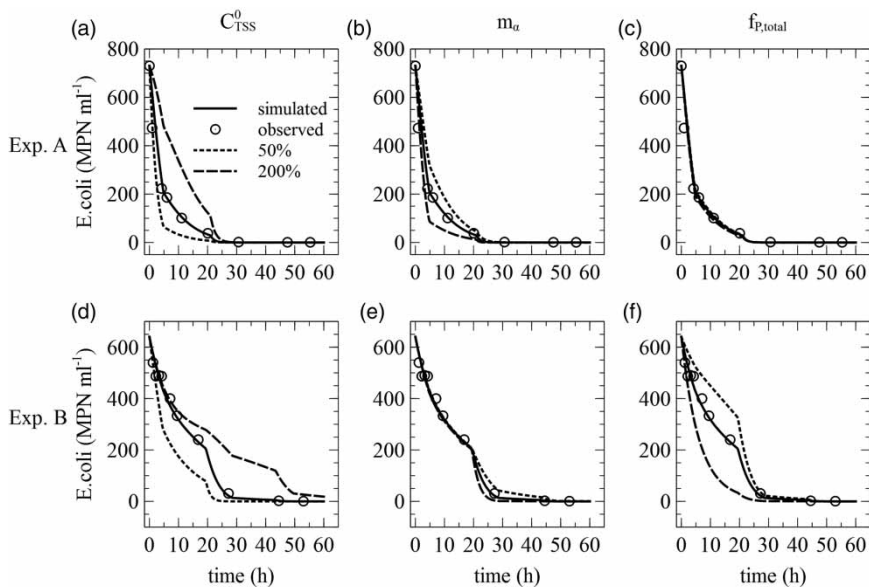
On the other hand, the UV intensity  $I_0$  and the UV sensitivity parameter  $m_\alpha$  had a stronger influence on Experiment A (ranked 5th and 6th) compared to Experiment B (ranked 11th and 12th). This indicates that *E. coli* is more susceptible to UV intensity variations, when less particulate matter is present, such as in Experiment A.

The impacts of three main parameters,  $C_{TSS}^0$ ,  $m_\alpha$ , and  $f_{P,total}$ , on the *E. coli* removal process have been assessed. Figure 5 illustrates the simulation results when these parameters were changed by 50% or 200% of the original parameter value. From Figure 5 it can clearly be seen that (i)  $C_{TSS}^0$  was sensitive in both experiments, (ii)  $m_\alpha$  was more sensitive in Experiment A where there was less TSS, and (iii)  $f_{P,total}$  was more influential in Experiment B.

## DISCUSSION

### Impact of two particle size classes on the accuracy of the model

There have been many attempts to investigate the influence of the settling velocity distribution on the sedimentation process in retention tanks and to incorporate more than one particle size class into particle (TSS) settling models (Huebner & Geiger 1996; Maruejols et al. 2012). However, there is still no



**Figure 5** | Assessment of the impacts of the major parameters  $C_{TSS}^0$ ,  $m_a$  and  $f_{P,total}$  on the *E. coli* removal process. Plots (a) to (c) refer to Experiment A and plots (d) to (f) refer to Experiment B. Measured values are average values of three samples and are indicated by circles.

application of it to the transport of FIB in aquatic water quality models. Some FIB transport models for flood events encounter overestimation of the FIB concentration directly after the hydraulic peak, followed by an underestimation (Jamieson *et al.* 2005; Gao *et al.* 2011; Ghimire & Deng 2013). This problem can be due to the incorporation of only one particle size class (constant settling velocity). In such cases, the settling velocity in the model may be lower than what actually occurs after the hydraulic peak where large particles and attached FIB are removed from the water column. Afterwards, the median settling velocity is overestimated as in reality the majority of particles remaining in the water column are small and settle slower.

In this study, the partitioning of particles into two particle size classes provided a more accurate and smooth removal prediction of *E. coli* compared to only one particle size class. According to section Influence of each mechanism on the removal of *E. coli*, the settling mechanism for *E. coli* removal is more influential when the TSS concentration increases. It can therefore be inferred that incorporating two particle size classes is definitely improving the prediction of FIB transport during CSOs when TSS concentrations are elevated (typically above  $100 \text{ mg l}^{-1}$ ).

#### Incorporation of TSS concentration-dependent parameters

This study attempted to find flexible equations for  $\alpha$  and  $k_d$  to fit FIB decay mechanisms at different TSS

concentrations.  $\alpha$  and  $k_d$  were  $8.5 \text{ m}^2 \text{ MJ}_{UV}^{-1}$  and  $0.047 \text{ h}^{-1}$ , respectively, when the initial TSS concentration was  $15 \text{ mg l}^{-1}$ . Sinton *et al.* (2002) obtained solar sensitivity coefficients ranging from 0.277 to  $0.395 \text{ m}^2 \text{ MJ}_{solar}^{-1}$  in fresh river water. Since they used solar intensity instead of UV intensity (3% of the total energy of sunlight is UV), their UV sensitivity coefficient would reach 9.1 to  $13.1 \text{ m}^2 \text{ MJ}_{UV}^{-1}$ , which is comparable to our reported values. They further found the dark die-off coefficients for *E. coli* to vary from 0.001 to  $0.043 \text{ h}^{-1}$ , which aligns well with our study ( $0.047 \text{ h}^{-1}$  when  $\text{TSS} = 15 \text{ mg l}^{-1}$ ), justifying the derived expressions for  $\alpha$  and  $k_d$ .

In this study it was assumed that as the TSS concentration increased, *E. coli* persistence in the water column would be augmented. The prolonged survival of enteric bacteria in river bed sediments compared to that in the water column has long been recognized (Sherer *et al.* 1992; Davies *et al.* 1995; Craig *et al.* 2002; Pachepsky & Shelton 2011). Likewise, for similar reasons the presence of suspended solids has also been linked with growth and survival of *E. coli* in the water column. Desmarais *et al.* (2002) found that adding sterile sediment to river water causes the *E. coli* population in the water column to regrow proportional to the amount of sediment added. Additionally, suspended particulate matter is also known to reduce the transmission of solar irradiation and therefore lowers the solar inactivation of microorganisms by shading and encasement (Dickenson & Sansalone 2012; Kollu &

Ormeci 2012). Whitby & Palmateer (1993) discovered a linear relationship between the TSS concentration and the number of FC in the effluent of wastewater treatment plants after UV disinfection. Hence, the correlation of the TSS concentration to the removal rate of bacteria as performed in this study is a necessity to achieve a precise model prediction of the FIB transport.

### *E. coli* removal mechanisms and patterns

Experiments A and B were dominated by different removal mechanisms (Figure 4). Figure 4(b)–4(d) show that natural and UV inactivation were the main removal mechanisms in Experiment A, because the sedimentation effect was very low throughout the whole experiment. Meanwhile, Figure 4(f)–4(h) indicate that the settling mechanism was the dominant removal mechanism during the first 20 hours in Experiment B, and afterwards natural and UV inactivation predominated.

Figure 4(a) and 4(e) additionally give insight into the different inactivation patterns of *E. coli* in Experiments A and B. Experiment A exhibited a fast removal for the first 30 hours and a tailing towards equilibrium at the end. Experiment B showed a lower removal during the first 20 hours followed by a fast removal. Blaustein et al. (2013) reported that most often the measured FIB decay patterns are of the type seen in Experiment A. The tailing in Experiment A is probably due to the fact that the carrying capacity of the environment is reached and the bacteria concentration remains at the background level (Easton et al. 2005; Blaustein et al. 2013). Furthermore, the inactivation patterns in wastewater are mostly the type observed in Experiment B (Blaustein et al. 2013), probably due to higher TSS concentrations and better nutrient supply. In this model, UV and natural inactivation rates were very high at the end of the experiment because of low TSS concentrations. In consequence the model did not perform well when the *E. coli* concentration began to level off and approached background concentrations less than 5 MPN ml<sup>-1</sup> (see Figure 4(a) and 4(e)). Since the focus of this study was to investigate the transport of FIB after CSOs, the prediction of such low MPN concentrations was not the main interest.

### CONCLUSIONS

The fate and transport of *E. coli* were investigated experimentally at large scale and subsequently described by a modelling approach. A 1D model was developed to predict

the TSS and *E. coli* concentrations under steady flow conditions throughout the experimental duration of 70 hours. The conclusions of the study are as follows.

1. Incorporating only a single particle size class results in an insufficient prediction of the TSS and *E. coli* concentrations in the water column. The model performs well when at least two particle size classes are included. The accuracy of the model could only be slightly improved by incorporating a third particle size class.
2. *E. coli* dynamics were considered to be dependent on the TSS concentration. The UV sensitivity coefficient  $\alpha$  and natural inactivation coefficient  $k_d$  were related to the TSS concentrations by an exponential expression. Thus, the model was capable of predicting *E. coli* concentrations accurately at different TSS levels. At high initial TSS concentrations ( $\geq 60$  mg l<sup>-1</sup>), incorporation of TSS-dependent  $\alpha$  and  $k_d$  resulted in an obvious improvement of the model performance. Constant  $\alpha$  and  $k_d$  values were sufficient to predict UV and natural inactivation when the initial TSS concentration was low.
3. Depending on the size and concentration of particles in the water column, *E. coli* removal is dominated by different removal mechanisms. The results revealed that the sedimentation process was more important under higher TSS concentrations. In contrast, bacteria were more susceptible to solar irradiance and other natural inactivation processes, when less suspended particulate matter was present.

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